

Cohesive sediment erosion in a combined wave-current boundary layer

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Key Points:

- Waves drive erosion by entraining sediment into the wave boundary layer.
- Tidal turbulence in the absence of waves is poorly correlated to erosion, except for larger relative depths.
- Boundary layer sediment fluxes are consistent with bed level measurements, laboratory flume studies, and previous field work.

Abstract

We conducted field work on the shoals of South San Francisco Bay to elucidate the mechanisms driving cohesive sediment erosion in a shallow, wave- and current-driven flow. Compiling data from three deployments, including measurements taken within the combined wave-current boundary layer, we found that waves were strongly correlated to turbulent sediment fluxes across all seasons and a range of deployment depths. Tidal turbulence was only correlated to turbulent sediment fluxes for larger relative depths, or when a wave-driven sediment flux into the boundary layer allowed the tidal shear stress to transport sediment into the overlying flow. Despite the dominance of waves in eroding sediment, we found favorable agreement between *in situ* boundary layer erosion measurements and laboratory erosion measurements conducted in a steady flume. Results were analyzed in the context of two benthic surveys which provided insight into the sediment bed properties.

Plain Language Summary

Marine sediments cover the majority of the Earth’s surface, and the movement of sediment through the environment affects water quality, coastal infrastructure, and the health of aquatic ecosystems. Sediments are primarily transported due to a combination of forces exerted by wave- and tidally-driven flows. However, the erosion (or picking up) of sediment from the sea floor by the flow is a complex process that occurs over very small spatial scales. Therefore, it is difficult to observe and is not particularly well-understood. In this paper, we use new measurement devices to observe how mud erodes in an estuary with varying wave and tidal conditions. We found that waves can effectively erode sediment into a thin region near the bed, allowing tidal currents to distribute the sediment throughout the rest of the water column. We compared these field erosion measurements with various other traditional measurement techniques and previous studies, and found general agreement among them. These results can be applied to improving computer models of sediment transport.

1 Introduction

Sediment erosion is a ubiquitous geophysical phenomenon that affects habitat restoration efforts, contaminated sediment remediation, and navigational dredging (Wood & Armitage, 1997; Van Maren et al., 2015). These processes are often simulated in numer-

ical sediment transport models. While coarse-grained sediments (e.g., quartz) can be treated as single particles to determine model parameters such as critical shear stress for erosion, estuarine sediments, which contain large percentages of minerals such as silt and clay can aggregate together to form suspended flocs and a continuum bed (Winterwerp & Van Kesteren, 2004). The characteristics of flocs make their erosion from the bed and subsequent vertical distribution in the water column difficult to predict. Therefore, many models rely on empirical parameterizations rather than first principles (Merritt et al., 2003; Papanicolaou et al., 2008).

Model parameterizations are often informed by field observations, which generally focus on the interplay between cohesive sediment dynamics and local hydrodynamics. In South San Francisco Bay, California, USA, the flow is driven by both tidal currents and short-period wind waves. This combination leads to nonlinear interactions between the wave and tidal turbulence near the bottom boundary. Numerous analytical models have been proposed to describe these dynamics (W. Grant & Madsen, 1979; Christoffersen & Jonsson, 1985; Coffey & Nielsen, 1987; You et al., 1991), and characteristics of the combined wave-current shear stress were analyzed for our study site in Egan et al. (2019). Connecting the hydrodynamics to erosion, field observations have shown that wave and current interactions significantly enhance sediment resuspension (Friedrichs et al., 2000; Brand et al., 2010; MacVean & Lacy, 2014). When waves erode sediment into the wave boundary layer, the mean current can more readily entrain sediment higher in the water column (Friedrichs et al., 2000).

Erosion has been studied in the laboratory as well. Flume studies have shown that the critical shear stress of the sediment bed increases with depth into the bed and the amount of time the bed consolidates (Mehta & Partheniades, 1982). These studies produced a semi-empirical erosion formula that varied exponentially with the normalized excess shear stress (i.e., excess applied stress relative to critical shear stress). Aside from exponential formulations, researchers have put forth power law (Lick, 1982; Maa et al., 1998) and linear erosion relationships (Sanford & Halka, 1993; Mei et al., 1997). One of the most widely used models is a unified erosion formulation validated on field measurements that describes both Type I erosion (depth-limited, i.e. shear forcing cannot keep up with increases in bed shear strength as erosion progresses) and Type II erosion (unlimited, i.e. erosion continues unimpeded because the shear forcing remains stronger than

bed shear strength) (Sanford & Maa, 2001). More complex models accounting for transient armoring, consolidation, and bioturbation have also been proposed (Sanford, 2008).

Due to the small spatial scales over which erosion occurs, it has historically been difficult to observe and quantify its driving mechanisms *in situ*. This has resulted in a relatively low-resolution understanding of a process that occurs over turbulent timescales in the millimeter-scale bottom boundary layer. Leveraging novel acoustic instrumentation, we simultaneously measured shear stress, sediment fluxes, and bed level within the combined wave-current boundary layer to elucidate the competing roles of waves and currents in eroding sediment from a muddy bed in South San Francisco Bay. This resulted in, to our knowledge, the first direct field measurements of a sediment flux coherent with the wave motion in an estuarine wave boundary layer. With data taken from three separate month-long deployments, we also analyzed seasonal variability in the erosive response to hydrodynamic forcing. We compared these results to more traditional flume-based laboratory methods of measuring erosion, and comment on the applicability of these results to erosion parameterizations in sediment transport models.

2 Methods

2.1 Field Deployment

We deployed five instrument platforms on the shallow, eastern shoals of South San Francisco Bay for three four-week periods: 07/17/2018 - 08/15/2018 (summer deployment), 01/10/2019 - 02/07/2019 (winter deployment), and 04/17/2019 - 05/15/2019 (spring deployment). Study sites covered a range of mean water depths (Table 1) and the deployment dates were selected to sample a diverse set of estuarine conditions in terms of wave strength and phytoplankton productivity. Platform 1 (P1) held three acoustic Doppler velocimeters (ADV) to measure turbulence and sediment fluxes throughout the water column, one profiling acoustic Doppler velocimeter (Vectrino) to measure turbulence and sediment fluxes in the bottom boundary layer, one acoustic Doppler current profiler (ADCP) to measure current profiles, and one RBR Bottom Pressure Recorder (BPR) to measure wave statistics. Approximately 30 meters from P1, we deployed an optical instrumentation platform (P1O) that contained two Sequoia Scientific Inc. LISST-100x's (LISST; Laser In-Situ Scattering and Transmissometry), which measured suspended sediment particle size distributions (PSDs). On platform 2 (P2) we deployed two ADVs, a LISST,

and a BPR. Platform 3 (P3) held an additional ADV, ADCP, BPR, and three optical backscatter sensors (OBS). During the summer campaign, we deployed platform 4a (P4a) which contained the same instrumentation as P3. For the winter and spring deployments, we moved this platform south to better capture wave propagation and the tidal pressure gradient, and renamed it platform 4b (P4b). Platform locations are shown in Figure 1, and instrumentation details and study site information are summarized in Table 1, which includes platform GPS coordinates, mean lower-low water level (MLLW), and instrument deployment height in centimeters above the bed (cmab).

Label	Location	MLLW (m)	Instrument	cmab
P1	37.58745°N, 122.18530°W	1.5	Vectrino Profiler	0 - 1.5
			ADV	5, 15, 45
			ADCP	15 - 400
			BPR	100
P1O	37.58730°N, 22.18530°W	1.5	LISST	15, 45
P2	37.58728°N, 122.17167°W	0.5	ADV	5, 15
			BPR	66
			LISST	35
P3	37.58550°N, 122.23141°W	2.5	ADV	15
			ADCP	15 - 400
			BPR	99
			OBS	15, 30, 80
P4a	37.58681 °N, 122.21182°W	2.25	Same as P3	
P4b	37.56130°N, 122.18530°W	2.25	Same as P3	

Table 1: Instrument platform details.

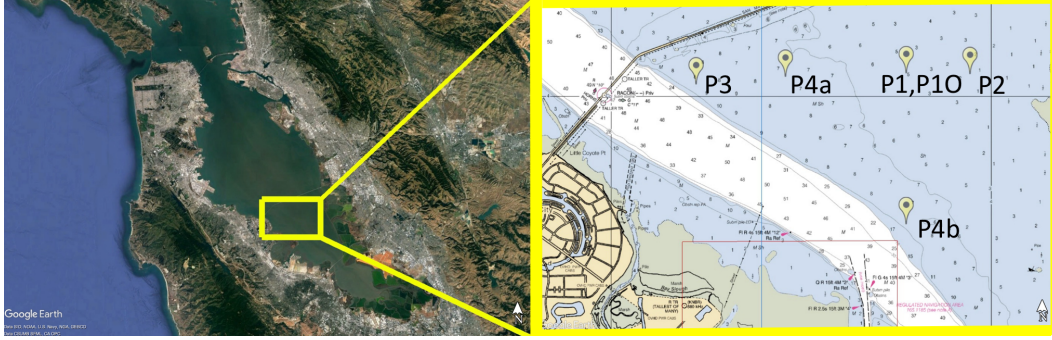


Figure 1: Study sites in South San Francisco Bay. P1, P1O, P2, and P3 were deployed in the summer, winter, and spring; P4a was only deployed in the summer, and was replaced by P4b in the winter and spring.

All ADVs were programmed to sample at 8 Hz for 14 minutes each hour, logging the pressure and 3D velocity. Each ADCP reported mean current profiles every 3 minutes based on 72 seconds of averaging. The BPRs logged pressure at 6 Hz for the entire deployment period, and each OBS reported turbidity every 5 minutes. The LISST at P1O collected a 60 second burst-averaged particle size distribution (PSD) every hour during the middle of the ADV sampling period, and the LISST at P2 measured a PSD every minute. The Vectrino was deployed with its measurement volume overlapping the bed such that it reported the 3D velocity at 64 Hz with 1 mm vertical resolution from 0 - 1.5 cmab for 12 minutes each hour in the summer, and 14 minutes each hour in the spring. Each of those deployments resulted in approximately two weeks of usable Vectrino data; after that point the measurement volume was either located too close to, or too far from the bed. The Vectrino collected no data during the winter because of a battery failure. Additional details about the deployment can be found in our previous paper analyzing the wave-current boundary layer dynamics at platform 1 (Egan et al., 2019).

In addition to deploying moored platforms, we collected two sediment box cores from each study site one day prior to the summer deployment. These cores were placed in a United States Environmental Protection Agency (USEPA) certified Sediment Erosion with Depth flume (SEDflume) to characterize erosion rates and critical shear stress with depth into the core (McNeil et al., 1996; Roberts et al., 1998). The sediment bulk density was also measured at each erosion depth interval.

During the winter and spring deployments, we conducted a sediment bed survey adjacent to P1 using an Ocean Imaging Systems Sediment Profile Imaging (SPI) camera (Rhoads & Germano, 1982, 1987). The SPI survey allowed for characterization of sediment bed properties at the sediment-water interface and 10–20 cm into the bed, along with biological activity through visual identification of sediment grain size, feeding voids, worm tubes, and burrows. Erosion data were analyzed in the context of these sediment bed characteristics.

2.2 SEDflume and SPI

Two SEDflume cores were collected from sites P1, P2, and P3 one day prior to the summer deployment. Core depths ranged from 31–52 cm, and each core had a cross-sectional area of $10 \times 15 \text{ cm}^2$. Cores were eroded with progressively increasing shear stresses of 0.1, 0.2, 0.3, 0.4, 0.6, 0.8, 1.2, 1.6, 3.2, and 6.4 Pa. The shear stress was increased to the next increment after either 10 minutes had passed, or 1 cm of sediment had eroded. The shear stress sequence was repeated for three 4-cm vertical sections into the core. In each section, the erosion data were fit to the power law formula

$$E = E_0 \tau^b, \quad (1)$$

where E_0 and b are empirical constants, and τ is the shear stress. The critical shear stress, τ_{cr} , was estimated as the shear stress corresponding to 0.5 mm of erosion in 10 minutes, i.e. the minimum detectable amount of erosion during a shear stress increment. Because we are focusing our analysis on near-bed erosion, we will only report critical shear stresses and erosion rates for the first of the three 4-cm eroded sections, which is most relevant for comparison to our field data.

SPI surveys were conducted during the winter and spring deployments on January 9, 2019 and again on May 7, 2019, in the area surrounding P1. A total of 11 locations were sampled in January and 19 were sampled in May, with two duplicate images collected at each location. The SPI system consists of a Nikon D7100 digital single-lens reflex camera with a 24.1-megapixel image sensor mounted inside an Ocean Imaging Model 3731 pressure housing system. The images are taken through a prism which penetrates up to 20 cm into the sediment bed, with each image resulting in a $14.5 \times 21 \text{ cm}^2$ profile view of the sediment-water interface. Camera settings were f10, ISO 400, and 1/60 shutter speed.

Image analysis was conducted using Integral Consulting Inc.’s MATLAB-based software, iSPI v1.1. The SPI image measured penetration depth, apparent redox potential discontinuity (aRPD) depth, grain size (in phi units), number of worm tubes on the surface, number of worm tubes at depth, number of feeding voids, burrows, and infaunal successional stage (Revelas et al., 2018). These parameters offered insight into the extent of biological activity at the bed and the vertical structure of bed properties, specifically in contrasting the relatively unconsolidated fluff layer at the bed with the firmer, more consolidated mud below.

2.3 Hydrodynamics

In South San Francisco Bay, the principal axis of the tidal ellipse runs approximately northwest to southeast, corresponding to the along-channel direction in Figure 1. Therefore, we rotated all ADV and Vectrino data into the coordinate system defined by the principal axes (i.e. the directions of maximum variance), as estimated by the ADCP mean velocity data at each of the platforms. The major component of velocity is denoted u , the minor component v , and the vertical velocity w .

Because the goal of this study was to examine the competing and synergistic roles of waves and currents in eroding sediment, it was necessary to define shear stresses that quantify the contribution of (a) currents alone, (b) waves alone, and (c) the combined action of the two. When the flow is both wave- and current-driven, the velocity can be decomposed as

$$u = \bar{u} + \tilde{u} + u', \quad (2)$$

where \bar{u} is the burst period-averaged velocity, \tilde{u} is the wave velocity, and u' is the turbulent fluctuating velocity. The presence of the wave velocity necessitates a wave-turbulence decomposition when estimating the turbulent Reynolds stress, $\overline{u'w'}$. For the ADV data, we chose the Benilov method (Benilov & Filyushkin, 1970), and for the Vectrino (which does not simultaneously log pressure), we used the phase method (Bricker & Monismith, 2007). The current-induced shear stress, τ_c , can then be defined in terms of the Reynolds stress magnitude:

$$\tau_c = \rho_0 |\overline{u'w'}|. \quad (3)$$

We can also define a wave-induced shear stress,

$$\tau_w = \frac{1}{2} \rho_0 f_w u_b^2, \quad (4)$$

where $\rho_0 = 1020 \text{ kg m}^{-3}$ is the fluid density, u_b is the bottom wave-orbital velocity, and the wave friction factor (Jonsson, 1967) is given by

$$f_w = 2\text{Re}_\delta^{-1/2}. \quad (5)$$

Here, Re_δ is the wave Reynolds number, defined as

$$\text{Re}_\delta = \frac{u_b \delta_w}{\nu}, \quad (6)$$

where $\delta_w = \sqrt{2\nu\omega^{-1}}$ is the Stokes wave boundary layer thickness and ν is the fluid kinematic viscosity. Equation 5 is valid for laminar wave boundary layers, and though the sediment beds at our study sites were often rough, the wave Reynolds number only reached $\mathcal{O}(10^2)$. Given the range of relative roughness values that we measured (see Section 3.3), wave Reynolds numbers of that magnitude could have induced a transitional wave boundary layer, but likely not a fully turbulent boundary layer (Jonsson, 1967; Lacy & MacVean, 2016). Therefore, we will use Equation 5 across all the study sites and deployments for consistency.

The bottom wave-orbital velocity, which was estimated following Wiberg and Sherwood (2008) as

$$u_b = \sqrt{2\text{var}(u')}, \quad (7)$$

can be evaluated using either ADV or Vectrino data. The magnitude of u_b , however, depends on the wave direction, which was not aligned with the major component of the tidal velocity. During the spring and summer deployments, diurnal northwesterly winds nearly always caused waves to propagate eastward. Therefore, we defined the dominant wave direction as the average wave direction over the deployment period at each platform, and estimated u_b based on the velocity in that direction. During the winter, wave activity was characterized primarily by storm events that drove both northward and eastward propagating waves. Because the directionality was less consistent, we defined the wave direction based on a nearest-neighbor interpolation to the dominant wave direction during a wavy burst period, which was defined as any burst period with $u_b > 0.05$.

Either the current shear stress (Equation 3) or the wave shear stress (Equation 4) would be reasonable inputs for the shear stress τ in our erosion formulation (Equation 1), but choosing one or the other results in significant differences in the fitting parameters, E_0 and b . A third choice for shear stress could include the effects of both waves

and currents, i.e. a combined wave-current shear stress

$$\tau_{wc} = \rho_0 u_{*wc}^2. \quad (8)$$

Here, u_{*wc} is the combined wave-current friction velocity estimated from the Grant-Madsen model (W. Grant & Madsen, 1979). We will calculate each of the above shear stress estimates (Equations 3, 4, and 8) using ADV data from each platform during all three deployments (and Vectrino data when available), and analyze the erosive response to the wave- and current-induced stresses. This will allow for identification of the dominant physical mechanisms that drive cohesive sediment erosion.

We will also examine erosion trends across a range of roughness regimes, parameterized by the bottom roughness, z_0 . To estimate z_0 , we first calculated a drag coefficient, C_D . This was approximated as the best-fit slope from a least-squares regression between the sign-preserving squared mean velocity at 15 cmab, $u|u|$, and the turbulent Reynolds stress at 15 cmab, $-\overline{u'w'}$. Assuming a logarithmic velocity profile, the bottom roughness can be estimated in terms of C_D as

$$z_0 = \frac{z_r}{\exp\left(\frac{\kappa}{\sqrt{C_D}}\right)}, \quad (9)$$

where $z_r = 15$ cmab is the reference height and $\kappa = 0.41$ is the von Kármán constant. The bottom roughness can then be related to the Nikuradse (or physical) bottom roughness with $k_b = 30z_0$. While the reference height, z_r , for this calculation is often taken as 1 meter above the bed (Dronkers, 1964), we believe that our 15 cmab ADV measurements provided a more reliable estimate of the mean velocity than the ADCP.

Wave statistics aside from u_b were estimated from BPR data. The wave frequency, ω , was estimated from the peak in the power spectral density of the BPR pressure signal during 14 minute segments corresponding to each ADV burst period. The power spectral density was calculated via the Welch method (Welch, 1967). The wave frequency was used to estimate the wavenumber, k , using the linear gravity wave dispersion relation, $\omega^2 = gk \tanh(kh)$, where g is acceleration due to gravity and h is the mean water depth. The wavenumber was used to calculate the relative depth, kh , and the wave frequency was used to calculate the bottom wave-orbital excursion,

$$A_b = \frac{u_b}{\omega}. \quad (10)$$

2.4 Sediment

Sediment data, including multiple metrics for erosion, were primarily derived from acoustic instruments. This was preferable to using raw optical measurements because the acoustic instruments provided co-located measurements of shear stress to which erosion must be related. Acoustic backscatter readings from the Vectrino and P1 ADVs were calibrated against water samples with known suspended sediment concentration (SSC) in the lab, using sediment collected from the study site. The calibration curves can be found in Egan et al. (2020). This method of SSC estimation has proven reliable for tracking relative changes in SSC over time at a single instrument (Brennan et al., 2002; Cartwright et al., 2013), though the precise SSC magnitude should be interpreted with caution because variations in acoustic transmit frequency and suspended sediment particle size can affect the acoustic backscatter amplitude used to infer SSC (Lohrmann, 2001). The P2 ADV acoustic backscatter data were calibrated against *in situ* LISST SSC measurements, which were derived by summing the LISST PSDs and multiplying by the sediment density measured in the lab. OBS turbidity data were calibrated against SSC samples in the lab (not shown), allowing for calibration of the P3 and P4a/b ADVs against *in situ* OBS SSC data. These calibration curves are shown in Figure 2.

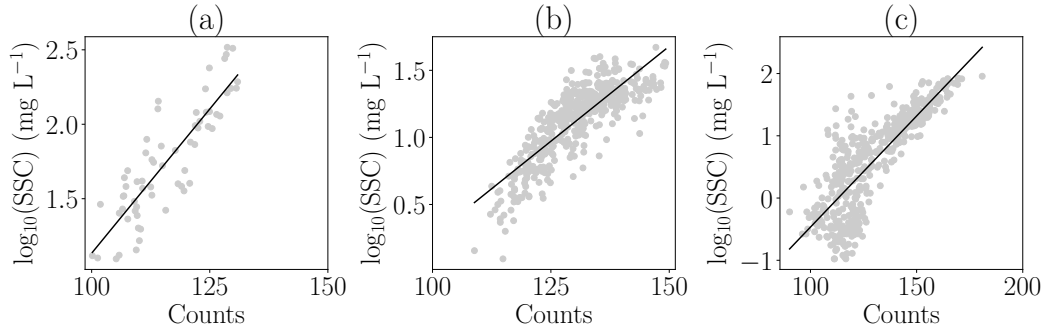


Figure 2: Acoustic backscatter (in instrument units of counts) to SSC calibration for 15 cmab ADVs at (a) Platform 2 ($r^2 = 0.75$), (b) Platform 3 ($r^2 = 0.69$), and (c) Platforms 4a and 4b ($r^2 = 0.67$), with the black line indicating a least squares fit.

In addition to providing the mean SSC, these data were used to estimate the turbulent sediment flux, $\overline{c'w'}$, which was calculated as the covariance between SSC and the vertical velocity for both the Vectrino and ADV data. For the Vectrino, this results in

a profile of $\overline{c'w'}$. We chose to vertically-average this profile between 0.3 and 0.7 cmab for a near-bed estimate; this will be the reported Vectrino turbulent sediment flux value for the remainder of the paper.

Previous work in South San Francisco Bay used the turbulent sediment flux as a proxy for erosion (Brand et al., 2010), and we will do the same with a slight modification. Because we measured the sediment bulk density at each study site, we can normalize the turbulent sediment flux by the sediment density, ρ_s , to obtain an erosion estimate

$$E = \frac{\overline{c'w'}}{\rho_s}. \quad (11)$$

This is advantageous because it gives erosion in units of m s^{-1} , allowing for direct comparison with SEDflume results after fitting to the power law erosion formula (Equation 1). For the 5 cmab ADV and Vectrino E measurements, we can additionally estimate a critical shear stress by the same metric used for the SEDflume data, i.e., finding the shear stress corresponding to 0.5 mm of erosion in 10 minutes, or $8.33 \times 10^{-5} \text{ cm s}^{-1}$.

We can measure erosion another way using the Vectrino bottom check feature. During each burst period, the Vectrino measured its distance from the nearest boundary. This was primarily used to calculate the vertical coordinates for velocity and SSC profiles, but bottom distance (BD) measurements can also be used to infer erosion rates. To that end, a fluctuating bed level, z_b , can be defined as

$$z_b = \overline{BD} - BD, \quad (12)$$

where \overline{BD} denotes the time-averaged bottom distance. This metric can be used as a proxy for erosion under the assumption that changes in z_b between bursts are due to sediment eroding or depositing beneath the instrument. That assumption may not always hold; for example, z_b could remain constant during erosive periods if the sediment under the Vectrino erodes at the same rate that the platform sinks into the bed. Platform consolidation is only expected to be significant immediately after deployment, however. Increases in z_b could also arise from transient clumps of sediment, flora, or fauna beneath the instrument, rather than from uniform deposition. These transient changes could be a substantial confounding factor, and will be considered when interpreting z_b data.

3 Results and Discussion

3.1 SPI survey

Across both the winter and spring surveys, the SPI data indicated that the survey area was fine grained ($> 4 \phi$ major mode) with both surface tube-dwellers and subsurface deposit feeders present. Both large polychaete and amphipod (likely *ampelisca*) tubes were seen at the sediment surface in a number of the images (e.g., Figure 3a). Evidence of stage 3 infauna (i.e., subsurface feeding voids, worms, or burrows) was observed in all but one of the images. Overall, the area surveyed in both the winter and spring appeared to be a relatively undisturbed, soft-bottom benthic habitat with a diverse benthic infaunal assemblage.

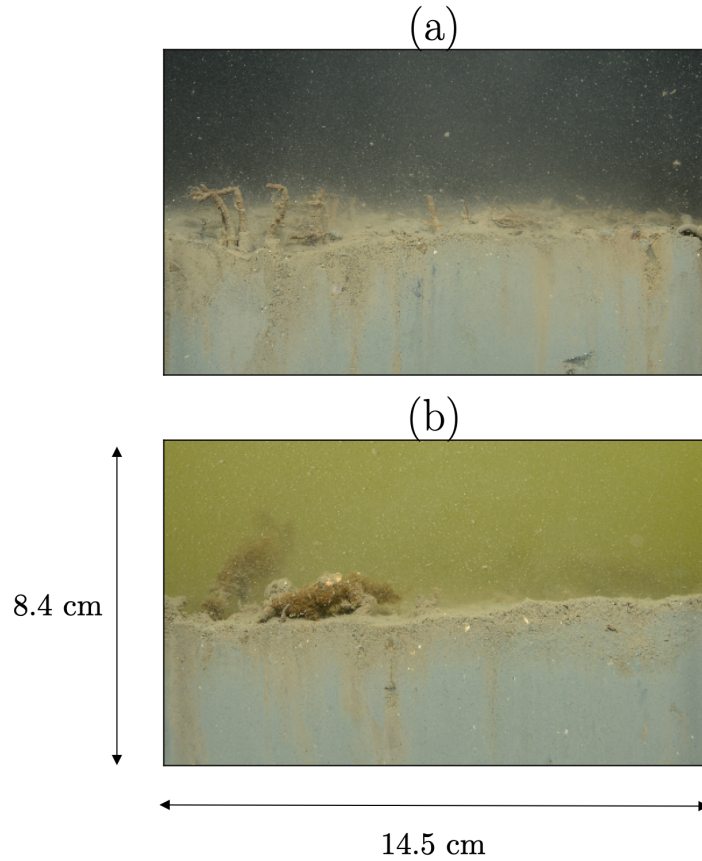


Figure 3: SPI camera images from the (a) winter survey and (b) spring survey. The dimensions of each image are $8.4 \text{ cm} \times 14.5 \text{ cm}$.

Table 2 shows the summary statistics for five of the parameters measured from the images (penetration and aRPD depths, and counts for surface tubes, feeding voids, and burrows) for the winter and spring data separately. Two-sample Kolmogorov-Smirnov tests were run to determine which of the six parameters were significantly different between surveys at a 5% significance level.

	Penetration	aRPD	Surface	Feeding	Burrow
	depth (cm)	depth (cm)	tube count	void count	count
Winter survey					
Avg	13.0	3.2	10	3	1
Min	8.8	1.5	1	1	0
Max	18.3	5.2	30	6	5
Spring survey					
Avg	10.0	1.1	9	3	1
Min	6.9	0.8	0	0	0
Max	11.3	1.5	20	5	3

Table 2: Summary statistics from the winter and spring SPI surveys. Bolded averaged are significantly different between surveys (Kolmogorov-Smirnov test, $p < 0.05$).

Both penetration depth, which averaged 13 cm in January and 10 cm in May, and aRPD depth, which averaged 3.2 cm in January and 1.1 cm in May, were significantly shallower during the spring compared to the winter. The aRPD depth reflects the interplay between near-surface bioturbation rates and labile organic matter inputs. This temporal trend may point to increased organic inputs to the sediment bed and higher sediment oxygen demand in the spring. In half of the May images, brownish/red algae was evident on the sediment surface (Figure 3b); this was not evident in January. Higher water temperatures and nutrient concentrations, along with higher microbial activity and levels of ambient light would contribute to this algal growth. Alternatively, or as a contributing factor, the seasonal difference in aRPD depths could reflect recent scouring of the sediment surface and the erosion of well-mixed, aerobic, unconsolidated surface sediments.

The reduced penetration depths in the spring suggest a firmer substrate in May than in January. This could reflect less intensive biogenic sediment mixing in the spring. However, there does not appear to be a major shift in community structure based on the lack of significant differences between the counts of the infauna themselves and their biogenic structures between January and May. Alternative explanations for the reduced prism penetration in May is the surface algal debris providing resistance to prisms descent into the sediment column and/or the recent erosion of the surface well-mixed layer, leaving more consolidated sediments in place. If the latter explanation is to blame for the reduced penetration depths, the spring erosion rates may be significantly reduced relative to winter given a constant bed shear stress. This potential change in bed composition will be revisited in the following sections as we analyze erosion data in the context of the SPI results.

3.2 Boundary layer sediment flux measurements

We can gain significant insight into the erosion dynamics at P1 by examining the near-bed response surrounding a large wind event. Plotted in Figure 4(a-c) are the bottom wave-orbital velocity, the near-bed erosive flux magnitude, and the bottom distance, respectively, for seven days of the spring deployment. Strong winds on 04/20 led to the largest bottom wave-orbital velocities that we measured during any of the deployments (Figure 4a), which were correlated to elevated sediment flux magnitudes (Figure 4b).

While the correlation between wave strength and turbulent sediment flux is not surprising, the unique aspect of this data set is the simultaneous measurement of bed level (Figure 4c). As the wave strength and sediment flux increased on 04/19, the bed level decreased, a direct measurement of erosion. Just before 04/20 00:00, z_b again increased, which may indicate deposition during the weak wave period. On 04/20, u_b and $|E|$ increase throughout the day, leading to 1.02 cm of erosion in terms of z_b from midnight 04/19 to midnight 04/20. This corresponds to an erosion rate of $1.18 \times 10^{-5} \text{ cm s}^{-1}$. As a comparison, Figure 4b gives $|E| = 1.36 \times 10^{-5} \text{ cm s}^{-1}$ averaged over the same period. These data are remarkably well-correlated, and lend confidence to both the SSC calibration of the Vectrino acoustic backscatter and the use of $\overline{c'w'}$ to estimate erosion in general.

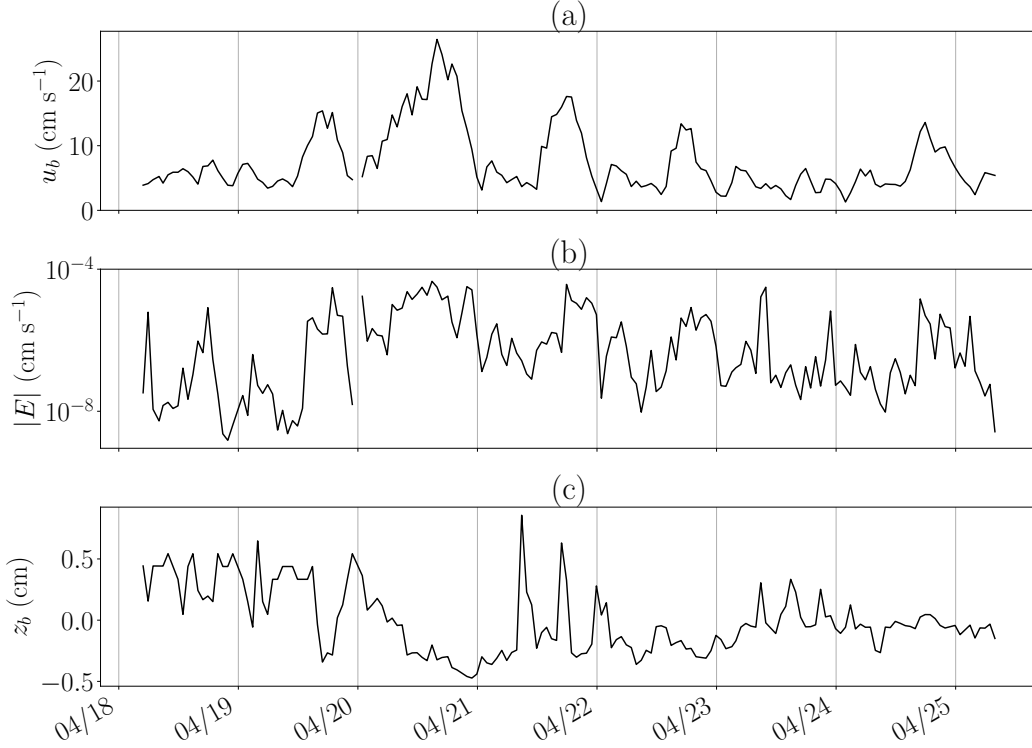


Figure 4: Seven day time series during the spring deployment showing Vectrino measurements of (a) the bottom wave-orbital velocity, u_b , (b) the erosive sediment flux magnitude, $|E|$ (Equation 11), and (c) the fluctuating bed level, z_b (Equation 12).

Once the strong winds subsided on 04/21, there were rapid fluctuations in z_b ; these could be due to either enhanced deposition after the storm or transient benthic flora/fauna beneath the Vectrino. Afterward, there was a relative decrease in mean bed level compared to the pre-storm period, from approximately 0.3 cm to -0.1 cm. This could indicate permanent erosion of part of the unconsolidated fluff layer seen in the SPI image (Figure 3b). The post-storm z_b signal also showed less variability with wave strength (Figure 5). For the three days prior to the storm, z_b decreased with wave shear stress as expected. Afterwards, z_b was on average much lower, and remained approximately constant even though the bed was subjected to wave shear stresses stronger than in the pre-storm period. This suggests that it had become more difficult to erode the bed, another indication that the exposed bed was then made up of more consolidated sediment rather than loose fluff. This interpretation is consistent with the SPI results presented in Section 3.1, where we hypothesized that a recent scouring event may have led to decreased

aRPD and prism penetration depth. It is possible that this storm was such an event, and that the unconsolidated fluff layer did not regain its original thickness by the time we conducted the SPI survey 2 weeks later.

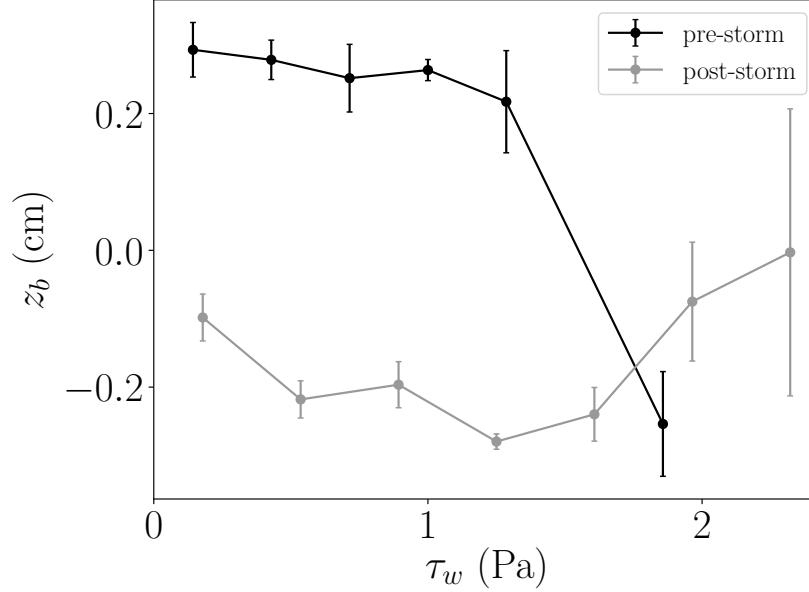


Figure 5: The fluctuating bed level, z_b , bin-averaged by the Vectrino-estimated wave shear stress, τ_w (Equation 4), for the three days prior to the 04/20 storm (black line) and three days afterward (gray line). Error bars denote the standard error on the bin-averaging.

Figure 6 shows the near-bed turbulent sediment flux measured by the Vectrino as a function of wave shear stress for the entire summer and spring deployments. The fit to the erosion parameterization (Equation 1) is denoted by the black line and has a coefficient of determination $r^2 = 0.45$. This was significantly higher than the fit to Equation 1 using $\tau = \tau_c$ (Equation 3), which was $r^2 = 0.08$. Using $\tau = \tau_{wc}$ (Equation 8) we obtained $r^2 = 0.21$. While all of these correlations are significant at the 95% confidence level for the number of data points used in the regression (771), the correlation is obviously much weaker for the current-induced shear stress. This implies that waves are the primary driver of near-bed sediment fluxes at this shallow study site, with tidally-driven turbulence playing a negligible role. This result is consistent with other studies in similar environments (Friedrichs et al., 2000; Brand et al., 2010).

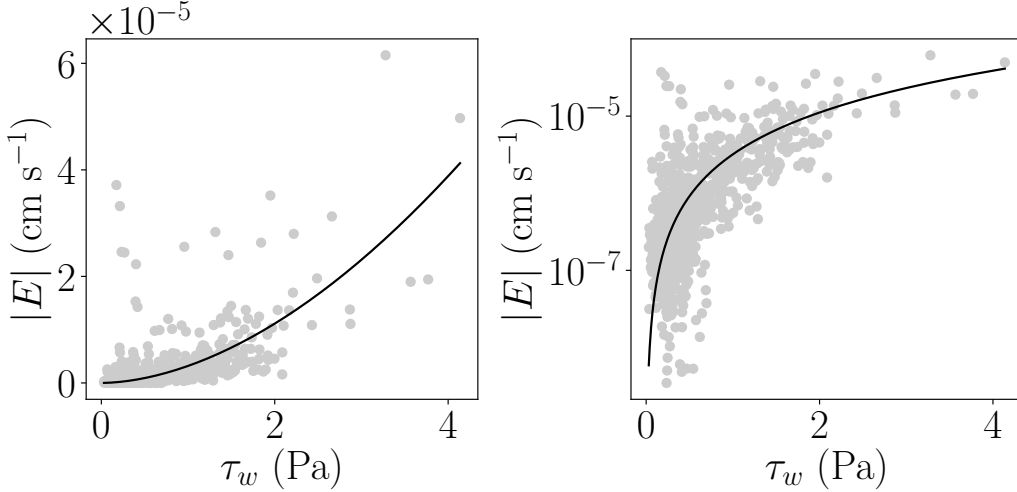


Figure 6: The erosive flux magnitude, $|E|$ (Equation 11) measured by the Vectrino and plotted against the wave shear stress, τ_w (Equation 4), in both (a) linear and (b) log scale. The black line denotes a fit to Equation 1.

Given that the erosive flux was best predicted by a wave shear-based parameterization, one might expect to find a strong wave peak in the c' power spectrum. Plotted in Figure 7 are power spectra for the Vectrino-measured SSC, averaged over every wavy burst period from the summer deployment. Spectra are shown at integer multiples of the Stokes wave boundary layer thickness, which was approximately $\delta_w = 1$ mm. The SSC spectra show a strong wave peak at $z = \delta_w$, but this peak decreases substantially by $z = 4\delta_w$, and nearly vanishes by $z = 6\delta_w$. At this last height, the power spectral density is greatest at lower frequencies and decays at higher frequencies, which is more indicative of a turbulence-dominated process. The spectral slope of the decay, however, scales as approximately f^{-1} , rather than $f^{-5/3}$ as theory predicts in the inertial subrange (G. K. Batchelor, 1959; G. Batchelor et al., 1959). An f^{-1} slope is predicted (and has been measured) in scalar spectra at frequencies above the inertial subrange (e.g. H. Grant et al., 1968), but the spectra in Figure 7 exhibit that slope over a much wider frequency band. There are numerous reasons that our data might vary from theory, namely that the measurements were taken in stratified, wavy conditions near a boundary, rather than homogeneous isotropic turbulence.

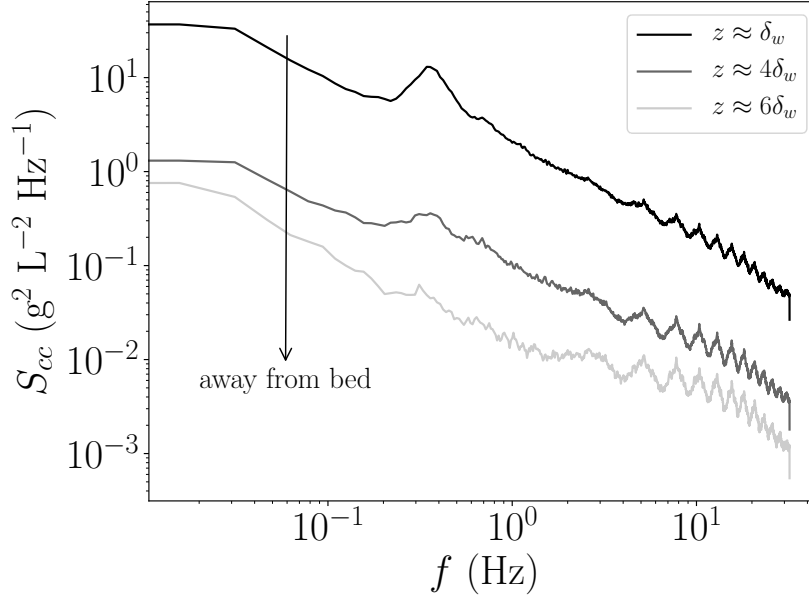


Figure 7: The power spectral density of (a) SSC, and (b) the Vectrino velocity in the dominant wave direction at heights of δ_w , $4\delta_w$, and $6\delta_w$ above the bed. Spectra are averaged over all wavy burst periods from the summer deployment.

These measurements show that wave-induced sediment resuspension is restricted to the wave boundary layer. Outside that region, waves merely oscillate a constant concentration back and forth along the path of the wave orbital. This can be explained physically as a consequence of settling in a short-period wind wave-dominated environment. The Stokes wave boundary layer thickness is approximately 1 mm for a three-second wave; assuming a floc settling velocity in the range $0.5\text{--}1\text{ mm s}^{-1}$, sediment particles that are eroded into the wave boundary layer settle back down into the fluff layer before they are resuspended by the next wave. These results also agree with Direct Numerical Simulations presented in Nelson and Fringer (2018), which showed that by a height of $6\delta_w$ above the bed in a combined wave-current flow, there was no wave phase variability in the suspended sediment signal.

From a scaling standpoint, the physical arguments presented above state that the vertical structure of the boundary layer suspended sediment profile is determined by a balance between the wave boundary layer thickness and a settling length scale. Defining a settling height $w_s T$ and letting the wave boundary layer thickness scale as $\delta \sim u_*/\omega$,

we find

$$\frac{w_s T}{\delta} \sim \frac{w_s / \omega}{u_{*w} / \omega} = \frac{w_s}{u_{*w}} \sim \frac{w_s}{f_w u_b} = \text{Ro}_w. \quad (13)$$

Here, u_{*w} is the wave-induced friction velocity, which scales as $f_w u_b$, and Ro_w is a wave Rouse number. For $\text{Ro}_w > 1$, vertical SSC gradients will only be strong near the bed, so wave stresses will not induce wave phase variability in SSC outside the wave boundary layer (e.g., Figure 7). For $\text{Ro}_w \ll 1$, sediment may be transported further upward by vertical wave velocities before settling back down to the bed.

Despite the lack of a wave peak in the measured SSC spectrum, the turbulent sediment flux outside the wave boundary layer remains highly correlated to the wave shear stress. This is because the waves erode sediment from the bed and suspend it in the wave boundary layer, thus allowing tidally-driven turbulence to induce turbulent sediment fluxes. In this sense, we can think of waves as a necessary but not sufficient forcing mechanism to transport sediment away from the bed. We can further examine this point by explicitly separating the wave-induced sediment flux from the turbulence-induced sediment flux via the phase method of Bricker and Monismith (2007). This allows for separation of the total vertical sediment flux, \overline{cw} , into its turbulence component, $\overline{c'w'}$, and its wave component, $\overline{c\tilde{w}}$. The turbulent sediment flux measured by the Vecrino can then be plotted as a function of the current shear stress, τ_c , and separated into cases of high and low wave sediment flux within the wave boundary layer. This is plotted in Figure 8, which incorporates all the Vecrino data from both the summer and spring deployments.

Across the range of τ_c , the turbulent sediment flux is significantly larger during burst periods with high \overline{cw} in the wave boundary layer. At the highest τ_c bin, the difference between the two fluxes is an order of magnitude, while at the lower τ_c bins, it is approximately a factor of five. Conceptually, the sediment suspended in the wave boundary layer can be thought of as new, more erodable bed for the tidal currents to erode. This effectively lowers the critical shear stress for erosion (note the increasing slope starting near $\tau_c \approx 0.05$ Pa), and increases the baseline erosion rate (i.e., E_0 in Equation 1).

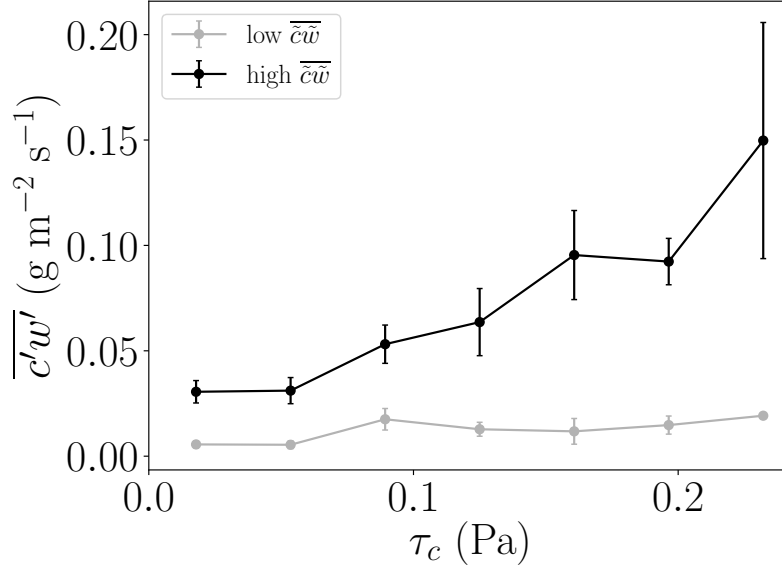


Figure 8: The turbulent sediment flux, $\overline{c'w'}$, plotted as a function of the current shear stress, τ_c , for the cases of low $\overline{c\tilde{w}}$ in the wave boundary layer (gray line), and high $\overline{c\tilde{w}}$ in the wave boundary layer (black line). High and low $\overline{c\tilde{w}}$ were defined as being, respectively, above and below a 60-hour fourth-order Butterworth low-pass filtered $\overline{c\tilde{w}}$ signal.

3.3 Spatial and seasonal variability

The strong correlation between wave strength and erosion is not universal across seasons and study sites. Figure 9 shows the turbulent sediment flux magnitude calculated by ADVs at 15 cmab at P2 (leftmost column), P1 (middle column) and P3 (rightmost column), corresponding to our shallow, moderate depth, and deepest site, respectively. We have neglected platforms 4a/b from this analysis because they were not deployed in the same location during each of the three seasons. The sediment flux is plotted against the wave shear stress (top row), current shear stress (middle row), and combined wave-current shear stress (bottom row). The fits to Equation 1 are denoted by the black line, with fitting parameters listed in Table 3. While each panel in Figure 9 shows combined data from all three deployments, the fitting parameters in Table 3 are separated by season. Confidence intervals (at a significance level $\alpha = 0.05$) were calculated for each of the fitting parameters using the bootstrap method, and are listed for each best-fit value of the erodability, E_0 , and the power, b . For the minimum number of data

points used in any of the regressions (379), the Pearson’s critical correlation coefficient at the 95% confidence level was $r^2 = 0.01$, so even the poorest fits were correlated enough to Equation 1 that a comparison of their fitting parameters was appropriate.

At P1 and P2, τ_c generally performed the worst in terms of r^2 for the Equation 1 fit. This is consistent across all three seasons, and in both the Vectrino and ADV data (the winter P1 ADV is the only exception). This is not true at P3, however, where the combined wave-current shear stress always performs worse than the current-induced shear stress. Platform 3 was the deepest study site, so it is reasonable to expect the stronger tidal currents to play an outsized role in eroding sediment compared to the shallower sites where wave-induced velocities do not decay as much with depth.

The summer and spring deployments were qualitatively similar in terms of wave- and current-induced erosion trends. The biggest differences were seen in the winter deployment, when waves were weakest. At P1 and P2 in the winter, τ_{wc} was a good predictor of erosion, whereas τ_w was the best choice at P3. The wave shear stress was the worst predictor at P1, despite being the best during the spring and summer deployments.

3.4 Nondimensional analysis

While the results in Table 3 offer insight into the factors controlling erosion at specific times and locations in San Francisco Bay, it is also useful to examine erosion trends as a function of nondimensional numbers that can be applied to other wavy flows. Specific nondimensional parameters that help to quantify the relative importance of waves and tidal currents include the depth-normalized significant wave height, $H_{sig}h^{-1}$, the wave-friction velocity ratio, $u_b u_*^{-1}$, the wave Reynolds number, Re_δ , the relative depth, kh , and the relative roughness, $k_b A_b^{-1}$. Each of these parameters was estimated for each 15 cmab ADV measurement burst period at platforms 1, 2, 3, and 4a/4b for all three deployments. We neglected burst periods where $u_b < 0.04 \text{ m s}^{-1}$ because it was difficult to reliably estimate a wave frequency (and thus, wavenumber) from the power spectra during these relatively weak wave periods. This resulted in a total of 4190 burst periods for analysis.

To quantify the capability of wave and current stresses to predict the turbulent sediment flux, we separated τ_c , τ_w , and $\overline{c'w'}$ into equally-sized bins sorted by each of the nondimensional parameters listed above. This binning was performed for individual ADVs

	E_0 (cm s ⁻¹ Pa ^{-b})			b (-)			r^2		
	P1	P2	P3	P1	P2	P3	P1	P2	P3
Summer									
τ_w	8.05e-07 ± 6.57e-08	6.13e-06 ± 1.14e-06	1.09e-06 ± 1.19e-07	0.94 ± 0.12	1.21 ± 0.71	1.63 ± 0.22	0.49	0.18	0.47
τ_c	1.54e-06 ± 6.20e-07	1.47e-05 ± 1.31e-05	2.22e-06 ± 1.89e-06	0.45 ± 0.14	0.33 ± 0.23	0.96 ± 0.41	0.18	0.03	0.29
τ_{wc}	1.77e-06 ± 3.21e-07	2.26e-05 ± 1.70e-05	1.31e-06 ± 4.17e-07	0.94 ± 0.15	1.26 ± 0.97	0.87 ± 0.23	0.46	0.17	0.24
Winter									
τ_w	3.73e-07 ± 1.06e-07	1.12e-05 ± 2.31e-06	1.91e-06 ± 2.77e-07	-0.21 ± 0.15	1.79 ± 0.42	1.12 ± 0.14	0.03	0.27	0.46
τ_c	1.91e-06 ± 1.02e-06	2.59e-05 ± 3.92e-05	6.28e-06 ± 2.89e-06	0.55 ± 0.22	0.68 ± 0.47	1.30 ± 0.30	0.14	0.06	0.38
τ_{wc}	2.27e-06 ± 1.07e-05	9.22e-05 ± 5.47e-05	2.98e-06 ± 2.40e-06	0.68 ± 1.38	1.95 ± 0.51	0.92 ± 0.56	0.25	0.26	0.22
Spring									
τ_w	1.40e-06 ± 1.98e-07	4.04e-06 ± 2.93e-06	1.53e-06 ± 2.95e-07	1.23 ± 0.17	2.74 ± 0.63	1.54 ± 0.47	0.62	0.53	0.38
τ_c	7.98e-06 ± 3.95e-06	3.26e-05 ± 3.53e-05	1.84e-06 ± 4.99e-07	1.15 ± 0.26	0.51 ± 0.48	0.73 ± 0.17	0.26	0.04	0.20
τ_{wc}	8.01e-06 ± 1.96e-06	1.01e-04 ± 2.55e-05	2.10e-06 ± 1.33e-06	1.48 ± 0.22	3.14 ± 0.94	0.87 ± 0.33	0.61	0.50	0.17

Table 3: Fitting parameters E_0 and b from Equation 1 for the 15 cmab ADVs at P1, P2, and P3 for the summer, winter, and spring deployments. Goodness-of-fit is indicated by the coefficient of determination (r^2) for each of the shear stresses (τ_c , τ_w , τ_{wc}) used in the regression.

to mitigate inter-instrument differences in the backscatter-SSC conversion. Equation 1 was fit to the measured $\overline{c'w'}$ using both τ_c and τ_w . The resulting r^2 and b were averaged across the different instruments for each nondimensional parameter bin. We did not analyze E_0 in this procedure because it depends too strongly on the specific acoustic backscatter-SSC calibration.

Out of all of the nondimensional numbers, we found that the relative depth, kh , and relative roughness, $k_b A_b^{-1}$, exerted the strongest control on the best-fit parameters, i.e., the variance in the erosion parameters was highest as a function of relative roughness and relative depth. In particular, r^2 responded most strongly to kh and the power b responded most strongly to $k_b A_b^{-1}$. For completeness, we will also show b as a function of kh and r^2 as a function of $k_b A_b^{-1}$. The results of this procedure are shown in Figure 10.

Figure 10a shows the evolution of the power, b , with the relative roughness, $k_b A_b^{-1}$. For both the wave and current shear stress, b remains relatively constant below $k_b A_b^{-1} \approx 1$, though the current shear stress value is significantly smaller than the wave shear stress value. As $k_b A_b^{-1}$ increases, however, both the wave and current shear stress values decrease substantially. This indicates that when roughness elements are significantly larger than the wave-orbital excursion, the flow, either wave- or current-driven, finds it much more difficult to erode sediment from the bed. Given the presence of dense canopies of tube worms and clams present at our study sites (which we noted during the SEDflume core collection and SPI surveys), this trend could be attributed to armoring effects by benthic fauna. The armoring could impede erosion by physical mechanisms, e.g., blocking the flow and reducing the shear stress, or biological mechanisms, e.g., increasing the concentration of sticky extracellular polymeric substances in the benthic sediments.

The evolution of r^2 with relative roughness (Figure 10b) closely tracks the trends in Figure 10a, with a slight increase in r^2 from low to moderate relative roughness, and a sharp decrease beyond $k_b A_b^{-1} \approx 1$. This implies that the erosion data in the roughest regime are inherently noisy, and do not respond strongly to increases in shear stress. An example of this type of correlation (or lack thereof) can be found in Figure 9d.

Examining erosion trends across relative depth, we find a much clearer delineation between a wave-controlled and a current-controlled regime. Both the power, b (Figure 10c), and r^2 (Figure 10d) are highest in the lowest kh bin, which includes measurements

in the shallow water wave limit. This implies that the wave shear stress is more effective than the current shear stress at eroding sediment in that regime, and a better predictor of the turbulent sediment flux. This is an intuitive result; we would expect that when wave orbitals remain constant with depth, they are more effective at eroding sediment. As kh increases through the intermediate depth range and into the deep water limit at the highest kh bin, the relative importance of the current-induced shear stress increases, and that of the wave-induced shear stress decreases before leveling off (in both b and r^2). The measurements at the highest kh bin are primarily from P3, the relatively deep platform where the current shear stress was a reasonable predictor of erosion across all three seasons.

The results in Table 3 and Figure 10 emphasize that it is critical to take local hydrodynamic conditions into account when parameterizing sediment transport in numerical models. Our data show that both kh and $k_b A_b^{-1}$ are particularly useful nondimensional numbers to consider when determining the dominant physical factors that influence erosion at a specific site.

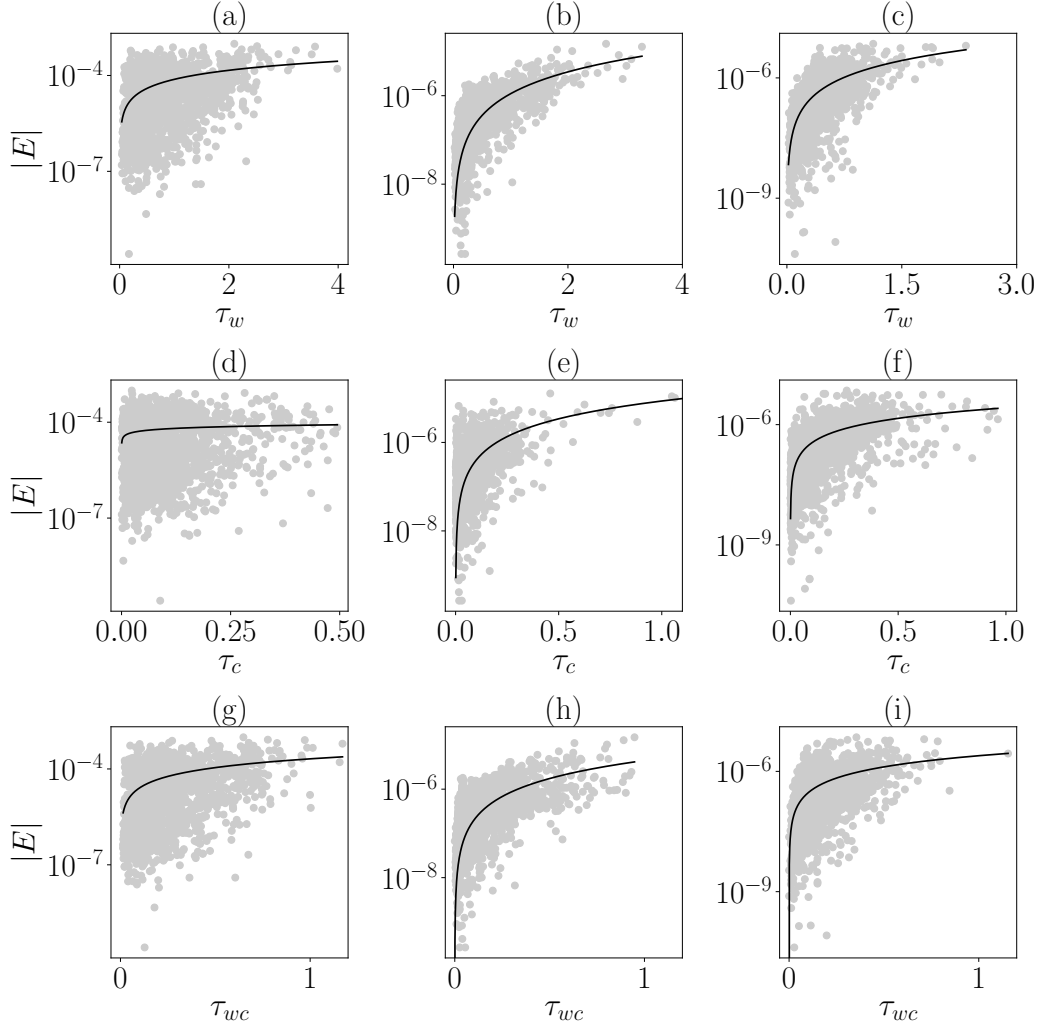


Figure 9: The erosive sediment flux magnitude, $|E|$ (Equation 11), calculated using 15 cmab ADV data from all three deployments and plotted against various shear stress estimates: (a) wave shear stress at platform 2, (b) wave shear stress at platform 1, (c) wave shear stress at platform 3, (d) current shear stress at platform 2, (e) current shear stress at platform 1, (f) current shear stress at platform 3, (g) wave-current shear stress at platform 2, (h) wave-current shear stress at platform 1, and (i) wave-current shear stress at platform 3. All shear stresses are in units of Pa, and all $|E|$ values are in units of cm s^{-1} . The black line denotes a fit to Equation 1.

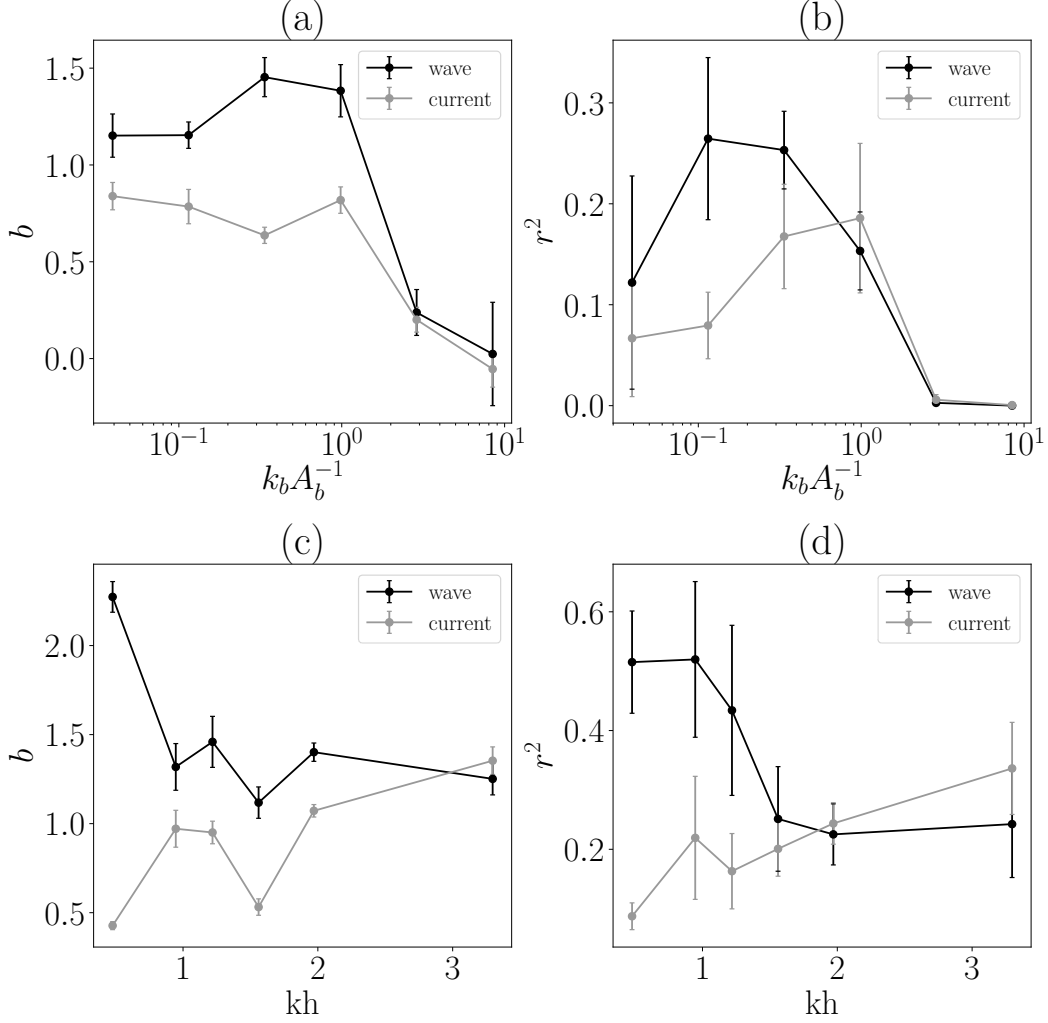


Figure 10: (a, c) The power, b , from a regression of 15 cmab ADV data to Equation 1, using the wave shear stress (black line) and current shear stress (gray line). Data are binned by (a) relative roughness, $k_b A_b^{-1}$, and (c) relative depth, kh , for individual instruments, and error bars denote the standard error on the regression, propagated through the average across the instruments. (b, d) The coefficient of determination, r^2 , estimated using the same regression procedure as in panels (a, c)

3.5 SEDflume and near-bed ADV comparison

One goal of this study was to compare SEDflume erosion measurements with *in situ* sediment flux measurements to assess whether laboratory measurements could adequately represent field conditions. To this end, we can compare the erodability, E_0 , and power, b , parameters estimated from a regression to Equation 1 for the SEDflume, 5 cmab ADV, and Vectrino data. We also estimated the critical shear stress, τ_{cr} , because the estimates are based on near-bed sediment flux measurements (we did not do this for the 15 cmab ADV data in Table 3). In general, E_0 describes the baseline magnitude of erosion, though inter-instrument comparisons of E_0 will be avoided because of differences in the acoustic backscatter calibration. The power, b , represents the erosive response to increased shear stress and is independent of the instrument-specific backscatter calibration. For this data set, the comparison between *in situ* velocimeter and SEDflume data comes with an additional caveat: wave shear stresses were primarily responsible for eroding sediment at our measurement sites in the summer. The SEDflume, conversely, applies a steady shear stress to the sediment bed. In each of these cases, we would expect fundamentally different mean shear and turbulence statistics for a given magnitude of mean flow, and thus, different mechanisms for erosion.

	E_0 (cm s ⁻¹ Pa ^{-b})	b (-)	τ_{cr} (Pa)	r^2
P1	3.74e-5	1.48	0.18	0.86
P2	6.52e-5	1.76	0.11	0.95
P3	5.02e-5	1.90	0.13	0.83

Table 4: Fitting parameters obtained through regression of Equation 1 to SEDflume erosion data.

Despite these differences, the exponential parameter, b , estimated from Vectrino and P1 ADV erosion data compared favorably with the P1 SEDflume value when using either the wave or combined wave-current shear stress (Table 4 vs Table 5). The critical shear stress values, on the other hand, were much larger when calculated using Vectrino and ADV data. The Vectrino and SEDflume showed closer agreement in τ_{cr} when using the combined wave-current shear stress, though they still differ by at least a fac-

tor of two. This is likely because of uncertainty in both the Vectrino backscatter calibration and the SEDflume testing. These confounding factors also affect the E_0 estimates, which were not well-correlated to the SEDflume E_0 , though the Vectrino τ_{wc} estimate was closest.

At platform 2, we found poor agreement between the SEDflume and ADV erosion estimates. This is largely due to the noisy P2 ADV data in the summer (note the extremely low r^2 values). The regressions were cleaner in the winter and spring and produced τ_{cr} estimates that agreed reasonably well with the SEDflume estimates. Given the high temporal variability of bed characteristics, however, these correlations should be interpreted with caution.

The results at platform 1 must also be analyzed in terms of temporal variability. One important difference between the SEDflume and velocimeter-derived erosion estimates is that the SEDflume cores were collected at a single point in time, while the Vectrino and ADV regressions were based on multiple weeks of data. This did not significantly affect the summer results; the velocimeters and SEDflume gave very similar erosion rate estimates. And while we do not have SEDflume data for the spring, the temporal variability of erosion rates can be analyzed in terms of the SPI survey data, which showed significantly reduced penetration depth in the spring compared to the winter. This clashes with E_0 and b estimates from the P1 ADV, which were higher in the spring compared to the winter. The SPI survey, however, occurred after the large storm event depicted in Figure 4, so it observed a less erodable bed. The ADVs and Vectrino, conversely, observed a highly erodable bed before the storm event, which biased the time-series estimates of erosion parameters. This highlights the time-varying nature of erosion rates and emphasizes the importance of including multilayer beds in sediment transport models.

3.6 Comparison to previous work

Finally, it is worthwhile to compare our erosion measurements to similar field studies. The most directly comparable are those conducted by Brand et al. (2010), and further analyzed in Brand et al. (2015), where Equation 1 was fit to ADV turbulent sediment flux measurements using a combined wave-current shear stress during a fall deployment and a spring deployment. Conditions in South San Francisco Bay are quite sim-

	E_0 (cm s ⁻¹ Pa ^{-b})			b (-)			τ_{cr} (Pa)			r^2		
	Vec	A1	A2	Vec	A1	A2	Vec	A1	A2	Vec	A1	A2
Summer												
τ_w	2.72e-06 ± 3.45e-07	8.80e-07 ± 7.20e-08	1.17e-05 ± 1.83e-06	1.45 ± 0.30	1.47 ± 0.23	0.76 ± 0.36	1.06	2.21	1.32	0.51	0.62	0.11
τ_c	8.16e-06 ± 1.28e-05	9.67e-07 ± 4.41e-07	2.70e-05 ± 9.16e-06	0.46 ± 0.29	0.24 ± 0.18	0.40 ± 0.17	-	-	1.64	0.08	0.04	0.11
τ_{wc}	9.93e-06 ± 4.53e-06	2.42e-06 ± 2.68e-07	2.77e-05 ± 1.14e-05	1.46 ± 0.49	1.21 ± 0.16	0.83 ± 0.41	0.43	1.86	0.37	0.46	0.46	0.11
Winter												
τ_w	-	7.76e-07 ± 2.51e-07	2.56e-05 ± 7.61e-06	-	0.50 ± 0.19	2.19 ± 0.90	-	-	0.17	-	0.21	0.61
τ_c	-	7.84e-07 ± 3.68e-07	9.03e-05 ± 1.09e-04	-	0.28 ± 0.15	1.13 ± 0.65	-	-	0.09	-	0.07	0.09
τ_{wc}	-	7.34e-07 ± 3.17e-07	2.83e-04 ± 2.34e-04	-	0.25 ± 0.13	2.21 ± 0.75	-	-	0.06	-	0.08	0.57
Spring												
τ_w	4.28e-06 ± 1.09e-06	1.34e-06 ± 1.27e-07	6.06e-06 ± 6.03e-06	1.59 ± 0.51	0.94 ± 0.11	2.51 ± 1.60	0.65	8.03	0.28	0.45	0.57	0.25
τ_c	1.80e-05 ± 1.09e-05	3.17e-06 ± 2.11e-06	2.47e-05 ± 2.66e-05	0.53 ± 0.16	0.56 ± 0.21	0.30 ± 0.42	1.82	-	5.92	0.08	0.13	0.01
τ_{wc}	2.44e-05 ± 1.27e-05	4.67e-06 ± 8.58e-07	1.28e-04 ± 1.52e-04	1.22 ± 0.34	1.07 ± 0.15	2.93 ± 1.98	0.27	1.47	0.09	0.30	0.52	0.25

Table 5: Fitting parameters to Equation 1 for the Vectrino (Vec) and 5 cmab ADVs at platforms 1 (A1) and 2 (A2). The critical shear stress, τ_{cr} (Pa), corresponds to 0.5 mm of erosion over 10 minutes. Estimated critical shear stress values $\tau_{cr} > 10$ were considered physically unrealistic and were removed from the table.

ilar in the fall and summer, so our summer data can be compared to their fall data. In terms of the power, b , Brand et al. (2015) estimated $b = 1.33 \pm 0.03$ during the fall, which was within the confidence bounds of our summer estimate, $b = 1.46 \pm 0.49$. The spring estimates are further from each other ($b = 2.03 \pm 0.06$ vs. $b = 1.22 \pm 0.34$), though our wave shear stress estimate did increase in the spring to $b = 1.59 \pm 0.51$. In terms of the erodability, E_0 , estimated values were within the error bounds of each other during both seasons. Despite the inherent noise in the data, it is encouraging that two sets of field studies conducted eight years apart in the same general sub-basin found comparable erosion rates. This implies that despite the high-frequency variability of hydrodynamic and sediment bed conditions, sediment transport models can achieve reasonable long-term accuracy with prudent choices for bulk erosion parameters.

4 Conclusions

Our analysis showed that waves are often the dominant driver of cohesive sediment erosion in shallow, wave- and current-driven flows. We found that the physical mechanism allowing waves to enhance resuspension is a “wave sediment flux”, analogous to the wave momentum flux, that is only measurable within and directly outside the wave boundary layer. To our knowledge, this is the first *in situ* measurement of the wave sediment flux in an estuarine bottom boundary layer, though previous field studies have hypothesized that sediment entrainment in the wave boundary layer allows for enhanced resuspension by tidal currents (Brand et al., 2010; MacVean & Lacy, 2014). This result also agrees with high resolution numerical simulations (Nelson & Fringer, 2018), and is qualitatively similar to sediment dynamics observed under lower frequency waves in wave-supported mud layers (e.g. Friedrichs et al., 2000; Hsu et al., 2009). Our results also emphasize the importance of using a shear stress that includes the effects of waves when parameterizing erosion in a sediment transport model. This is especially important when the flow is within a lower relative depth regime, where we found that tidally-driven turbulence plays a negligible role in inducing turbulent sediment fluxes.

We also presented *in situ* sediment flux measurements within the wave-current boundary layer, which showed general agreement with erosion measurements taken in a more traditional sediment flume, and with ADVs placed further from (though still close to) the bed. The trends in our sediment flux measurements, specifically the relatively strong and weak responses to wave forcing before and after a storm-induced erosion event, are

consistent with measurements of the relatively unconsolidated fluff layer that we imaged during the SPI survey. These results also emphasize the importance of considering scour history when parameterizing cohesive sediment erosion.

Taken together, the benthic survey, SEDflume data, and boundary layer flux measurements paint a comprehensive picture of an estuarine sediment bed subjected to various degrees of wave and tidal stresses. Given their consistency with SEDflume data, the Vectrino boundary layer measurements show particular promise for characterizing the *in situ* response to these hydrodynamic forcing mechanisms, especially when coupled with bed level observations. The simultaneous measurement of high resolution wave and turbulence data are particularly valuable for informing erosion parameterizations in cohesive sediment transport models.

Acknowledgments

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